

Raw (unfactored) loads

MODEL

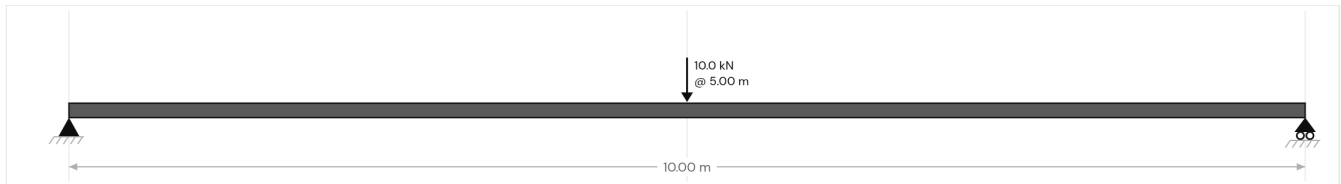
Span **10.00 m** Section **250UB31.4** E **200.00 GPa** I_x **$44.50 \times 10^6 \text{ mm}^4$** A **$4010.00 \text{ mm}^2$**
 Z **$354.00 \times 10^3 \text{ mm}^3$**

Support	Location
Pinned	0.00 m
Roller	10.00 m

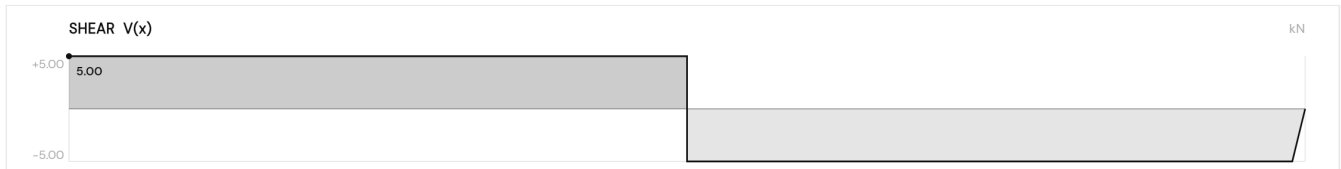
Load	Detail
Point	10.00 kN @ 5.00 m

DIAGRAMS

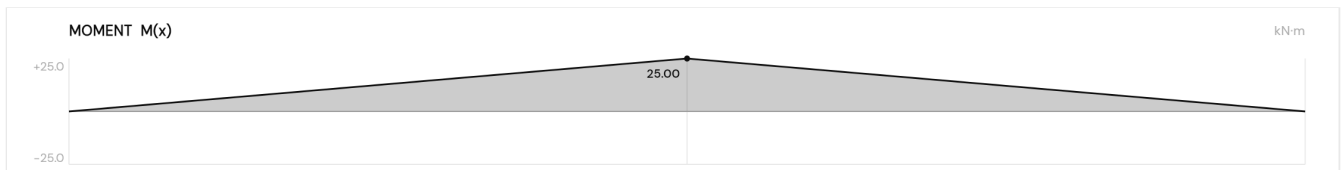
BEAM SCHEMATIC



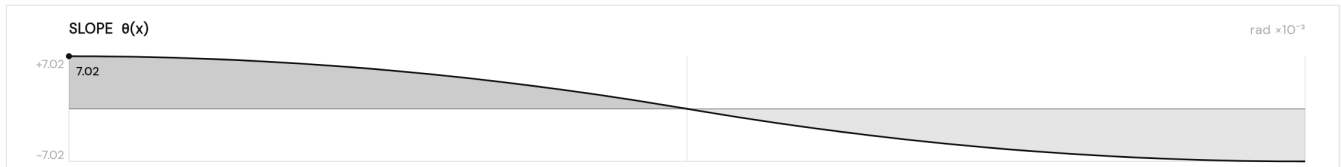
SHEAR



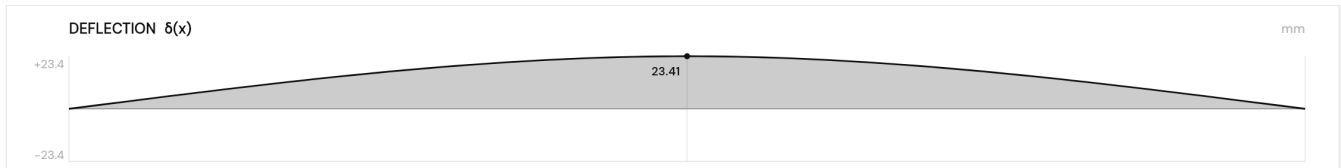
MOMENT



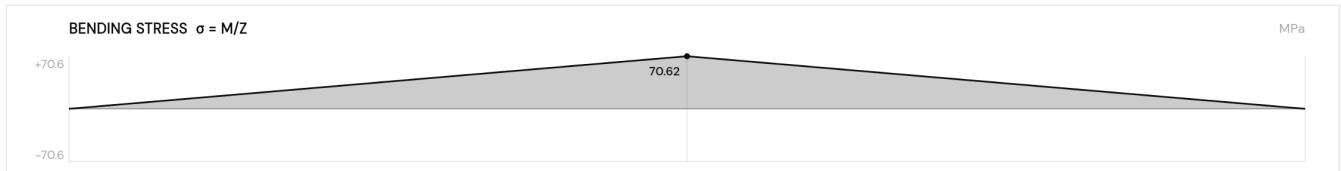
SLOPE



DEFLECTION



BENDING STRESS



AVERAGE SHEAR STRESS



REACTIONS

Location	Vertical	Moment
0.00 m	5.00 kN	—
10.00 m	5.00 kN	—

MAXIMA

Quantity	Value
Max shear $ V $	5.00 kN
Max moment $ M $	25.00 kN·m
Max deflection $ \delta $	23.408 mm
Max bending stress $ \sigma $	70.62 MPa
Max shear stress $ \tau $	3.25 MPa

DESIGN CHECK · AS 4100

$f_y = 300$ MPa, $\phi = 0.9$.

Check	Demand / Capacity	Utilisation	Verdict
Bending $M / \phi M_s$	25.00 / 95.58 kN·m	26%	OK
Shear $V / \phi V_v$	5.00 / 249.03 kN	2%	OK
Deflection $\delta / (L/250)$	23.41 / 40.00 mm	59%	OK

NOTES & LIMITATIONS

- Solver: CivilKit Beams v0.4.0 — direct stiffness, Euler–Bernoulli (transverse only). Validated against 103 closed-form / verification cases. mageengineering.com.au/civilkit-beams
- Design check: AS 4100 — Steel structures. Section moment capacity $\phi M_s = \phi \cdot Z_e \cdot f_y$ (cl 5.2) and section shear capacity $\phi V_v = \phi \cdot 0.6 \cdot f_y \cdot A_w$ (cl 5.11), assuming a compact bisymmetric section. Member moment capacity ϕM_b (lateral–torsional buckling, cl 5.6) and combined–actions checks are not yet included.
- No plastic modulus S on this section — Z_e is taken as Z , so the reported ϕM_s is conservative (low by ~10–15% for typical I-sections).
- Section data: indicative — properties should be cross-checked against the relevant national standard and a current manufacturer catalogue before professional use.
- Inclined loads: only the vertical component is analysed; axial components are reported in the warnings and ignored by this transverse-only solver.